



## ENERGY DISSIPATORS - Design of different types (1-7) of energy dissipation arrangements as Per USBR guidelines

As per USBR (UNITED STATES BUREAU OF RECLAMATION) various energy dissipation arrangements guidelines have been given.

A tremendous amount of experimental as well as theoretical work has been performed in connection with the hydraulic jump on a horizontal apron.

General Investigation of the hydraulic jump on Horizontal Apron (Basin I).

- The objectives of the study were
- ① to determine the applicability of the hydraulic jump formula for the entire range of conditions experienced in design.
  - ② To determine the length of the jump over the entire practical range
  - ③ To observe catalog & evaluate the various forms of the jump.

### ① The Froude number

$$F_1 = \frac{V_1}{\sqrt{gD_1}}$$

$F_1$  = is a dimensionless parameter

$V_1$  = velocity of flow

$D_1$  = depth of flow

$g$  = acceleration due to gravity

### ② Application of hydraulic Jump formula

The expression for the hydraulic jump based on pressure-momentum may be written as

$$D_2 = -\frac{D_1}{2} + \sqrt{\frac{D_1^2}{4} + \frac{2V_1^2 D_1}{g}}$$

or

$$D_2 = -\frac{D_1}{2} + \sqrt{\frac{D_1^2}{4} + \frac{2V_1^2 D_1^2}{gD_1}}$$

where  $D_1$  &  $D_2$  are the depths before & after the jump.

These depths are often called conjugate or sequent depths.

### ③ Length of the Jump

The length of the jump is plotted in two ways

The first method can be got from the table ① page ⑤ of USBR code.



## Forms of Hydraulic Jump

When the Froude no is unity, the water is flowing at critical depth, thus a jump cannot form. This corresponds to point O on the specific energy diagram. For the values of Froude no b/w 1 to 1.7 there is a slight difference in  $D_1$  &  $D_2$ .

- ① Water flowing under a sluice gate discharges into a rectangular stilling basin the same width as the gate. The average velocity & the depth of the flow after contraction of jet is  $V_1 = 85$  ft per sec.,  $D_1 = 5.6$  feet. Determine the conjugate tail water depth (i.e.  $D_2$ ), length of basin required to confine the jump.

Soln

$$F_1 = \frac{V_1}{\sqrt{g D_1}} = \frac{85}{\sqrt{9.8 \times 5.6}} = 11.46$$

With the value of Froude no 11.46 in table ① p. 48

$$\frac{D_2}{D_1} = 9.45$$

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$$D_2 = 9.45 \times 5.6 = 52.92 \text{ feet}$$

in column ②  $\frac{L}{D_2}$  for Froude no 11.46

$$\frac{L}{D_2} = 6.05$$

$$L = 6.05 \times D_2$$

$$= 6.05 \times 52.92$$

$$L = \underline{\underline{320 \text{ feet}}}$$

length of basin necessary is  $L = \underline{\underline{320 \text{ feet}}}$

## ii. Stilling basin for high dam & Earth dam spillways & large canal structures (Basin ii)

It is possible to reduce the jump loss by the installation of accessories such as baffles & sills in the stilling basin. These accessories exert a stabilizing effect & also increase factor of safety. This type of stilling basins have been used on high dams, & earth dam spillways & large canal structures. This basin

- contains
- ① Chute blocks at the upstream end &
  - ② A dented sill at the downstream end.

The following rules are recommended for generalization of Basin ii

- ① An additional factor of safety is advisable for both low & high values of Froude no. A minimum margin of factor of safety of 5% of  $D_2$  is recommended.



- ② Basin II may be effective down to a flood no of 4 but the lower values should not be taken for gented.
- ③ The length of basin can be obtained from the intake curve.
- ④ The height of chute blocks is equal to the depth of flow entering the basin or  $D_1$ .
- ⑤ The width & spacing should be equal to approximately  $D_1$ .
- ⑥ The height of the dentated still is equal to  $0.2D_2$ , & the maximum width & spacing recommended is approximately  $0.15 D_2$ .

### III Stilling basin for canal structures, small outlet works & small spillways (Basin-III)

A shorter basin having a simpler end sill may be used if baffle piers are placed downstream from the chute blocks. The most effective way to shorten a stilling basin is to modify the jump by the addition of appurtenances in the basin.

- ① Basin - III has a large factor of safety against jump sweepout & operates equally well for all values

of the flood no above 4.

- ② Basin ii should not be used where baffle piers will be exposed to velocities above the 50 to 60 feet per second.
- ③ The height, width & spacing of the chute blocks are equal to  $D$ , the same as Basin-ii.
- ④ The height of baffle piers increases with the flood no as can be observed from column 22 Table (4) - pg 36 USBR code
- ⑤ The most effective position of the baffle piers is  $0.8 D_2$  downstream from the chute blocks.
- ⑥ The height of the solid end sill is also shown to vary with the flood no, although there is nothing critical about this dimension.
- ⑦ Stilling Basin iii may be effective for values of the flood no as low as 4.



## IV Stilling Basin design for Canal structures & Diversion Dams (Basin IV)

The characteristics of the hydraulic jump for Froude no b/w 2.5 & 4.5 and the design of an adequate stilling basin designated as Basin - IV. Low dams & outlet works fall in this category.

- ① The length of basin IV is relatively short and can be obtained by appu curve.
- ② No baffle piers are needed in the basin as they will prove a greater detriment than aid.
- ③ The addition of small triangular sill placed at the end of the apron for scow control is desirable.
- ④ The following expression can be used for computing the length of basins:-

$$L = \frac{Q}{C N \sqrt{2g}}$$

$Q$  = total discharge in c.f.s

$C$  = experimental co-efficient

$S$  = width of a space in feet

$N$  = Number of spaces

$g$  = acceleration due to gravity

$y =$  is the depth of the flow