

Since the torque transmitted is the product of the tangential load and the radius of the pulley, therefore tangential load acting on the pulley

$$= \frac{T}{R} = \frac{238 \times 10^3}{600} = 396.7 \text{ N}$$

Since the pulley has four arms, therefore tangential load on each arm,

$$W = 396.7/4 = 99.2 \text{ N}$$

and maximum bending moment on the arm,

$$M = W \times R = 99.2 \times 600 = 59\,520 \text{ N-mm}$$

Let $2b$ = Minor axis in mm, and

$$2a = \text{Major axis in mm} = 2 \times 2b = 4b \quad \dots(\text{Given})$$

∴ Section modulus for an elliptical cross-section,

$$Z = \frac{\pi}{4} \times a^2 b = \frac{\pi}{4} (2b)^2 \times b = \pi b^3 \text{ mm}^3$$

We know that bending stress (σ_b),

$$15 = \frac{M}{Z} = \frac{59\,520}{\pi b^3} = \frac{18\,943}{b^3}$$

or $b^3 = 18\,943/15 = 1263$ or $b = 10.8 \text{ mm}$

∴ Minor axis, $2b = 2 \times 10.8 = 21.6 \text{ mm}$ **Ans.**

and major axis, $2a = 2 \times 2b = 4 \times 10.8 = 43.2 \text{ mm}$ **Ans.**

5.5 Bending Stress in Curved Beams

We have seen in the previous article that for the straight beams, the neutral axis of the section coincides with its centroidal axis and the stress distribution in the beam is linear. But in case of curved beams, the neutral axis of the cross-section is shifted towards the centre of curvature of the beam causing a non-linear (hyperbolic) distribution of stress, as shown in Fig. 5.8. It may be noted that the neutral axis lies between the centroidal axis and the centre of curvature and always occurs within the curved beams. The application of curved beam principle is used in crane hooks, chain links and frames of punches, presses, planers etc.

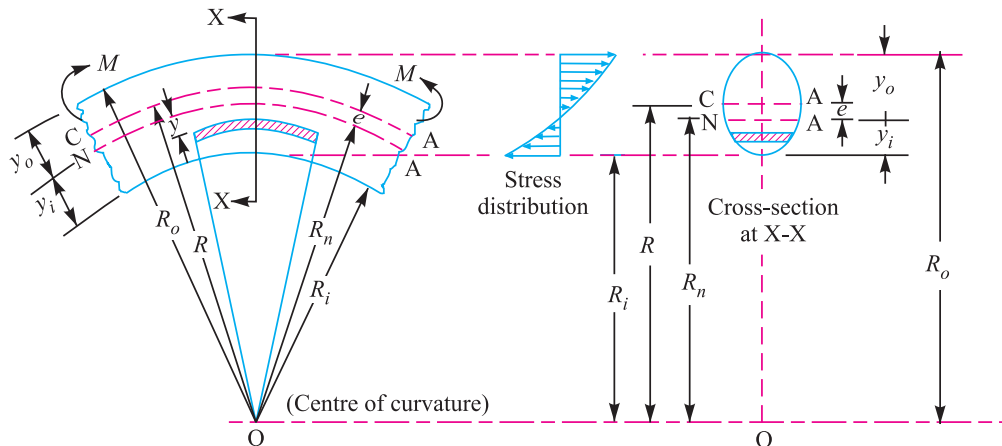


Fig. 5.8. Bending stress in a curved beam.

Consider a curved beam subjected to a bending moment M , as shown in Fig. 5.8. In finding the bending stress in curved beams, the same assumptions are used as for straight beams. The general expression for the bending stress (σ_b) in a curved beam at any fibre at a distance y from the neutral

axis, is given by

$$\sigma_b = \frac{M}{A \cdot e} \left(\frac{y}{R_n - y} \right)$$

where

M = Bending moment acting at the given section about the centroidal axis,

A = Area of cross-section,

e = Distance from the centroidal axis to the neutral axis = $R - R_n$,

R = Radius of curvature of the centroidal axis,

R_n = Radius of curvature of the neutral axis, and

y = Distance from the neutral axis to the fibre under consideration. It is positive for the distances towards the centre of curvature and negative for the distances away from the centre of curvature.

Notes : 1. The bending stress in the curved beam is zero at a point other than at the centroidal axis.

2. If the section is symmetrical such as a circle, rectangle, I-beam with equal flanges, then the maximum bending stress will always occur at the inside fibre.

3. If the section is unsymmetrical, then the maximum bending stress may occur at either the inside fibre or the outside fibre. The maximum bending stress at the inside fibre is given by

$$\sigma_{bi} = \frac{M \cdot y_i}{A \cdot e \cdot R_i}$$

where

y_i = Distance from the neutral axis to the inside fibre = $R_n - R_i$, and

R_i = Radius of curvature of the inside fibre.

The maximum bending stress at the outside fibre is given by

$$\sigma_{bo} = \frac{M \cdot y_o}{A \cdot e \cdot R_o}$$

where

y_o = Distance from the neutral axis to the outside fibre = $R_o - R_n$, and

R_o = Radius of curvature of the outside fibre.

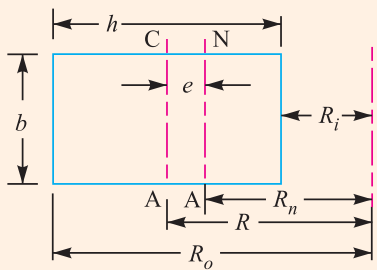
It may be noted that the bending stress at the inside fibre is *tensile* while the bending stress at the outside fibre is *compressive*.

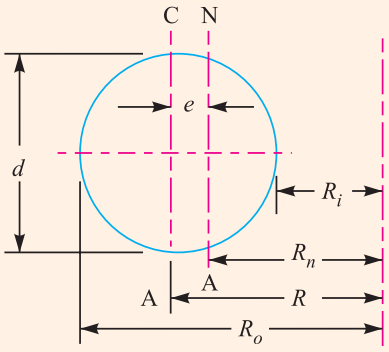
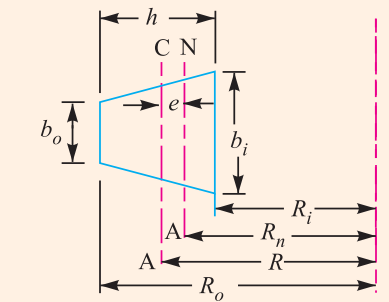
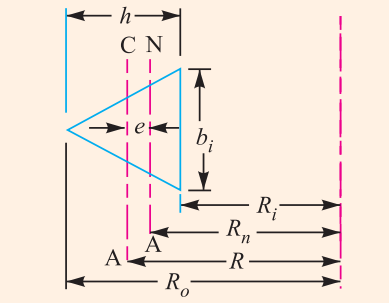
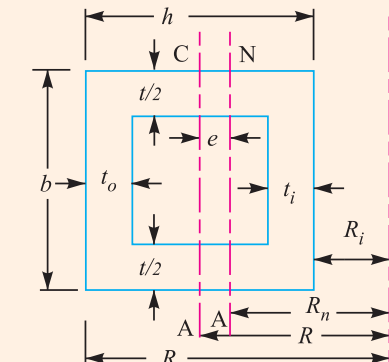
4. If the section has an axial load in addition to bending, then the axial or direct stress (σ_d) must be added algebraically to the bending stress, in order to obtain the resultant stress on the section. In other words,

Resultant stress, $\sigma = \sigma_d \pm \sigma_b$

The following table shows the values of R_n and R for various commonly used cross-sections in curved beams.

Table 5.2. Values of R_n and R for various commonly used cross-section in curved beams.

Section	Values of R_n and R
	$R_n = \frac{h}{\log_e \left(\frac{R_o}{R_i} \right)}$ $R = R_i + \frac{h}{2}$

Section	Values of R_n and R
	$R_n = \frac{[\sqrt{R_o} + \sqrt{R_i}]^2}{4}$ $R = R_i + \frac{d}{2}$
	$R_n = \frac{\left(\frac{b_i + b_o}{2}\right) h}{\left(\frac{b_i R_o - b_o R_i}{h}\right) \log_e \left(\frac{R_o}{R_i}\right) - (b_i - b_o)}$ $R = R_i + \frac{h(b_i + 2b_o)}{3(b_i + b_o)}$
	$R_n = \frac{\frac{1}{2} b_i \times h}{\frac{b_i R_o}{h} \log_e \left(\frac{R_o}{R_i}\right) - b_i}$ $R = R_i + \frac{h}{3}$
	$R_n = \frac{(b-t)(t_i + t_o) + t \cdot h}{b \left[\log_e \left(\frac{R_i + t_i}{R_i}\right) + \log_e \left(\frac{R_o}{R_o - t_o}\right) \right] + t \cdot \log_e \left(\frac{R_o - t_o}{R_i + t_i}\right)}$ $R = R_i + \frac{\frac{1}{2} h^2 \cdot t + \frac{1}{2} t_i^2 (b-t) + (b-t) t_o \left(h - \frac{1}{2} t_o\right)}{h t + (b-t)(t_i + t_o)}$

and radius of curvature of the centroidal axis,

$$R = R_i + \frac{h(b_i + 2b_o)}{3(b_i + b_o)} = 25 + \frac{40(18 + 2 \times 6)}{3(18 + 6)} \text{ mm}$$

$$= 25 + 16.67 = 41.67 \text{ mm}$$

Distance between the centroidal axis and neutral axis,

$$e = R - R_n = 41.67 - 38.83 = 2.84 \text{ mm}$$

and the distance between the load and centroidal axis,

$$x = 100 + R = 100 + 41.67 = 141.67 \text{ mm}$$

∴ Bending moment about the centroidal axis,

$$M = W \cdot x = 5000 \times 141.67 = 708\,350 \text{ N-mm}$$

The section at X-X is subjected to a direct tensile load of $W = 5000 \text{ N}$ and a bending moment of $M = 708\,350 \text{ N-mm}$. We know that direct tensile stress at section X-X,

$$\sigma_t = \frac{W}{A} = \frac{5000}{480} = 10.42 \text{ N/mm}^2 = 10.42 \text{ MPa}$$

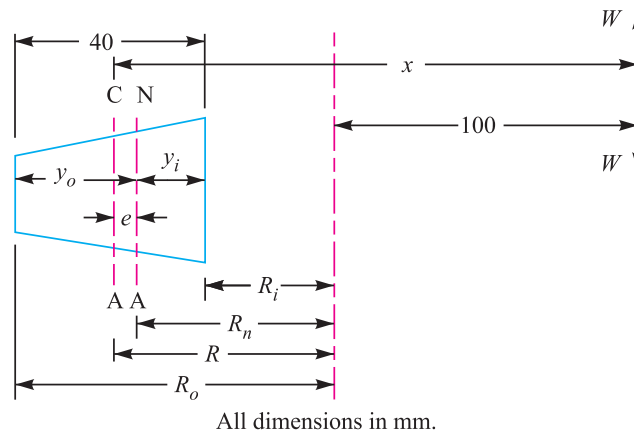


Fig. 5.10

Distance from the neutral axis to the inner surface,

$$y_i = R_n - R_i = 38.83 - 25 = 13.83 \text{ mm}$$

Distance from the neutral axis to the outer surface,

$$y_o = R_o - R_n = 65 - 38.83 = 26.17 \text{ mm}$$

We know that maximum bending stress at the inner surface,

$$\sigma_{bi} = \frac{M \cdot y_i}{A \cdot e \cdot R_i} = \frac{708\,350 \times 13.83}{480 \times 2.84 \times 25} = 287.4 \text{ N/mm}^2$$

$$= 287.4 \text{ MPa (tensile)}$$

and maximum bending stress at the outer surface,

$$\sigma_{bo} = \frac{M \cdot y_o}{A \cdot e \cdot R_o} = \frac{708\,350 \times 26.17}{480 \times 2.84 \times 65} = 209.2 \text{ N/mm}^2$$

$$= 209.2 \text{ MPa (compressive)}$$

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∴ Resultant stress on the inner surface

$$= \sigma_t + \sigma_{bi} = 10.42 + 287.4 = 297.82 \text{ MPa (tensile) Ans.}$$

and resultant stress on the outer surface,

$$= \sigma_t - \sigma_{bo} = 10.42 - 209.2 = -198.78 \text{ MPa}$$

$$= 198.78 \text{ MPa (compressive) Ans.}$$



A big crane hook

Example 5.11. The crane hook carries a load of 20 kN as shown in Fig. 5.11. The section at X-X is rectangular whose horizontal side is 100 mm. Find the stresses in the inner and outer fibres at the given section.

Solution. Given : $W = 20 \text{ kN} = 20 \times 10^3 \text{ N}$; $R_i = 50 \text{ mm}$; $R_o = 150 \text{ mm}$; $h = 100 \text{ mm}$; $b = 20 \text{ mm}$

We know that area of section at X-X,

$$A = b.h = 20 \times 100 = 2000 \text{ mm}^2$$

The various distances are shown in Fig. 5.12.

We know that radius of curvature of the neutral axis,

$$R_n = \frac{h}{\log_e \left(\frac{R_o}{R_i} \right)} = \frac{100}{\log_e \left(\frac{150}{50} \right)} = \frac{100}{1.098} = 91.07 \text{ mm}$$

and radius of curvature of the centroidal axis,

$$R = R_i + \frac{h}{2} = 50 + \frac{100}{2} = 100 \text{ mm}$$

∴ Distance between the centroidal axis and neutral axis,

$$e = R - R_n = 100 - 91.07 = 8.93 \text{ mm}$$

and distance between the load and the centroidal axis,

$$x = R = 100 \text{ mm}$$

∴ Bending moment about the centroidal axis,

$$M = W \times x = 20 \times 10^3 \times 100 = 2 \times 10^6 \text{ N-mm}$$

The section at X-X is subjected to a direct tensile load of $W = 20 \times 10^3$ N and a bending moment of $M = 2 \times 10^6$ N-mm. We know that direct tensile stress at section X-X,

$$\sigma_t = \frac{W}{A} = \frac{20 \times 10^3}{2000} = 10 \text{ N/mm}^2 = 10 \text{ MPa}$$

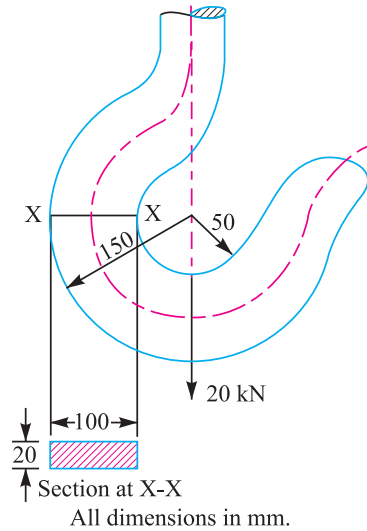


Fig. 5.11

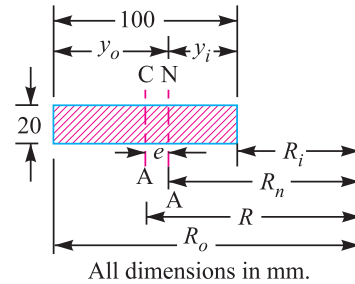


Fig. 5.12

We know that the distance from the neutral axis to the inside fibre,

$$y_i = R_n - R_i = 91.07 - 50 = 41.07 \text{ mm}$$

and distance from the neutral axis to outside fibre,

$$y_o = R_o - R_n = 150 - 91.07 = 58.93 \text{ mm}$$

∴ Maximum bending stress at the inside fibre,

$$\sigma_{bi} = \frac{M \cdot y_i}{A \cdot e \cdot R_i} = \frac{2 \times 10^6 \times 41.07}{2000 \times 8.93 \times 50} = 92 \text{ N/mm}^2 = 92 \text{ MPa (tensile)}$$

and maximum bending stress at the outside fibre,

$$\begin{aligned} \sigma_{bo} &= \frac{M \cdot y_o}{A \cdot e \cdot R_o} = \frac{2 \times 10^6 \times 58.93}{2000 \times 8.93 \times 150} = 44 \text{ N/mm}^2 \\ &= 44 \text{ MPa (compressive)} \end{aligned}$$

∴ Resultant stress at the inside fibre

$$= \sigma_t + \sigma_{bi} = 10 + 92 = 102 \text{ MPa (tensile) Ans.}$$

and resultant stress at the outside fibre

$$= \sigma_t - \sigma_{bo} = 10 - 44 = -34 \text{ MPa} = 34 \text{ MPa (compressive) Ans.}$$

Example 5.12. A C-clamp is subjected to a maximum load of W , as shown in Fig. 5.13. If the maximum tensile stress in the clamp is limited to 140 MPa, find the value of load W .

Solution. Given : $\sigma_{t(max)} = 140 \text{ MPa} = 140 \text{ N/mm}^2$; $R_i = 25 \text{ mm}$; $R_o = 25 + 25 = 50 \text{ mm}$; $b_i = 19 \text{ mm}$; $t_i = 3 \text{ mm}$; $t = 3 \text{ mm}$; $h = 25 \text{ mm}$

We know that area of section at X-X,

$$A = 3 \times 22 + 3 \times 19 = 123 \text{ mm}^2$$

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The various distances are shown in Fig. 5.14. We know that radius of curvature of the neutral axis,

$$R_n = \frac{t_i (b_i - t) + t \cdot h}{(b_i - t) \log_e \left(\frac{R_i + t_i}{R_i} \right) + t \log_e \left(\frac{R_o}{R_i} \right)}$$

$$= \frac{3(19 - 3) + 3 \times 25}{(19 - 3) \log_e \left(\frac{25 + 3}{25} \right) + 3 \log_e \left(\frac{50}{25} \right)}$$

$$= \frac{123}{16 \times 0.113 + 3 \times 0.693} = \frac{123}{3.887} = 31.64 \text{ mm}$$

and radius of curvature of the centroidal axis,

$$R = R_i + \frac{\frac{1}{2} h^2 \cdot t + \frac{1}{2} t_i^2 (b_i - t)}{h \cdot t + t_i (b_i - t)}$$

$$= 25 + \frac{\frac{1}{2} \times 25^2 \times 3 + \frac{1}{2} \times 3^2 (19 - 3)}{25 \times 3 + 3(19 - 3)} = 25 + \frac{937.5 + 72}{75 + 48}$$

$$= 25 + 8.2 = 33.2 \text{ mm}$$

Distance between the centroidal axis and neutral axis,

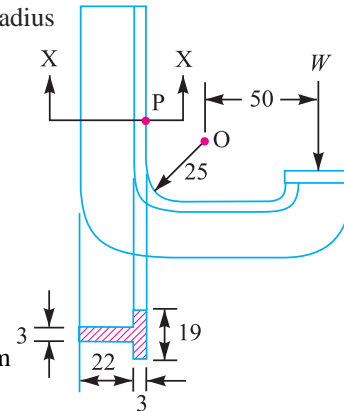
$$e = R - R_n = 33.2 - 31.64 = 1.56 \text{ mm}$$

and distance between the load W and the centroidal axis,

$$x = 50 + R = 50 + 33.2 = 83.2 \text{ mm}$$

∴ Bending moment about the centroidal axis,

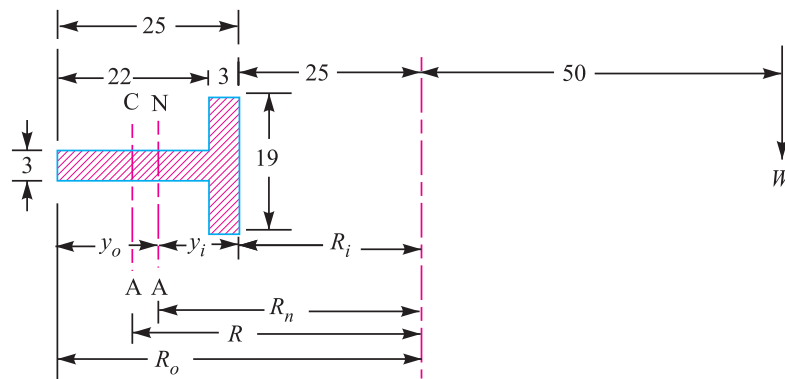
$$M = W \cdot x = W \times 83.2 = 83.2 \text{ W N-mm}$$



Section of X-X

All dimensions in mm.

Fig. 5.13



All dimensions in mm.

Fig. 5.14

The section at X-X is subjected to a direct tensile load of W and a bending moment of $83.2 W$. The maximum tensile stress will occur at point P (i.e. at the inner fibre of the section).

Distance from the neutral axis to the point P ,

$$y_i = R_n - R_i = 31.64 - 25 = 6.64 \text{ mm}$$

Direct tensile stress at section X-X,

$$\sigma_t = \frac{W}{A} = \frac{W}{123} = 0.008 W \text{ N/mm}^2$$

and maximum bending stress at point P,

$$\sigma_{bi} = \frac{M \cdot y_i}{A \cdot e \cdot R_i} = \frac{83.2 W \times 6.64}{123 \times 1.56 \times 25} = 0.115 W \text{ N/mm}^2$$

We know that the maximum tensile stress $\sigma_{t(max)}$,

$$140 = \sigma_t + \sigma_{bi} = 0.008 W + 0.115 W = 0.123 W$$

$$\therefore W = 140/0.123 = 1138 \text{ N Ans.}$$

Note : We know that distance from the neutral axis to the outer fibre,

$$y_o = R_o - R_n = 50 - 31.64 = 18.36 \text{ mm}$$

\therefore Maximum bending stress at the outer fibre,

$$\sigma_{bo} = \frac{M \cdot y_o}{A \cdot e \cdot R_o} = \frac{83.2 W \times 18.36}{123 \times 1.56 \times 50} = 0.16 W$$

and maximum stress at the outer fibre,

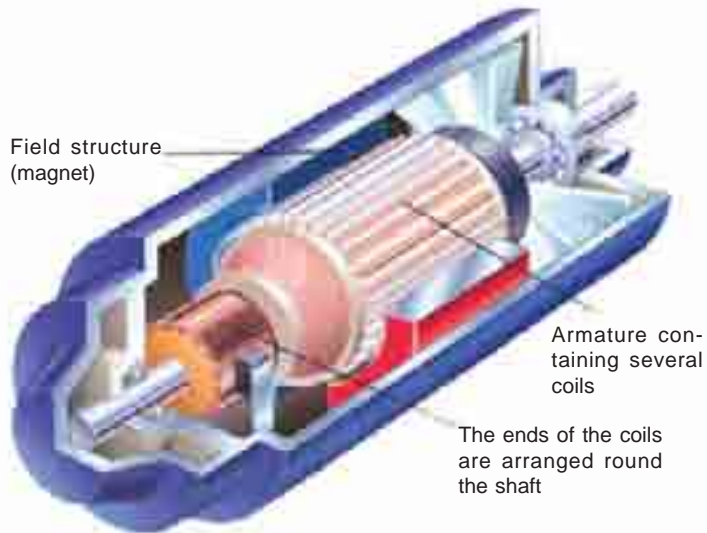
$$\begin{aligned} &= \sigma_t - \sigma_{bo} = 0.008 W - 0.16 W = -0.152 W \text{ N/mm}^2 \\ &= 0.152 W \text{ N/mm}^2 \text{ (compressive)} \end{aligned}$$

From above we see that stress at the outer fibre is larger in this case than at the inner fibre, but this stress at outer fibre is compressive.

5.6 Principal Stresses and Principal Planes

In the previous chapter, we have discussed about the direct tensile and compressive stress as well as simple shear. Also we have always referred the stress in a plane which is at right angles to the line of action of the force.

But it has been observed that at any point in a strained material, there are three planes, mutually perpendicular to each other which carry direct stresses only and no shear stress. It may be noted that out of these three direct stresses, one will be maximum and the other will be minimum. These perpendicular planes which have no shear stress are known as **principal planes** and the direct stresses along these planes are known as **principal stresses**. The planes on which the maximum shear stress act are known as planes of maximum shear.



Big electric generators undergo high torsional stresses.